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I, **ASIT SHUKLA**, certify that the work embodied in this thesis is my own bonafide work and carried out by me under the supervision of **Professor Piyush Rai** from **July-2014 to December-2020**, at the **Department of Mining Engineering**, Indian Institute of Technology (BHU), Varanasi. The matter embodied in this thesis has not been submitted for the award of any other degree/diploma.

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## *Acknowledgement*

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*I gratefully acknowledge the support and guidance of my thesis supervisor Prof. Piyush Rai, Department of Mining Engineering, IIT (BHU). Without his thoughtful encouragement and careful supervision, this thesis would never have taken shape. I am blessed to have spent time in their tutelage.*

*I also extend sincere thanks to RPEC members of my committee, Dr. Mohd. Zaheer Khan Yusufzai and Prof. Suprakash Gupta for his valuable suggestions and feedback on my research, and encouragement at various stages of the research work.*

*I would like to give a special thanks to Ashwani Sonkar, Dr. Hira Lal Yadav Sir, Shah Fateh Azam, Shailendra Chawla, and Mr. Rizwan Hasim for being wonderful friends for their help and support given to me during completion of this research work.*

*I would like to express my deep sense of gratitude to my parents, family members and relatives for their unwavering support and encouragement.*

*At last, but not least, thanks be to God for my life through all tests in the past years. You have made my life more bountiful.*

(ASIT SHUKLA)

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## **LIST OF ABBREVIATIONS**

<b>AISC</b>	American Institute of Steel Construction
<b>CEN</b>	European Committee for Standardization
<b>CIDECT</b>	International Committee for the Development and Study of Tubular Structures
<b>CHS</b>	Circular Hollow Section
<b>RHS</b>	Rectangular Hollow Section
<b>SHS</b>	Square Hollow Section
<b>CISC</b>	Canadian Institute of Steel Construction
<b>EC</b>	Eurocode
<b>EN</b>	European Norm
<b>NCL</b>	Northern Coalfields Limited

## SYMBOLS

### Uppercases

$A_0$	cross-sectional area of the chord member
$A_v$	shear resistant area of an element
$E$	Young modulus or modulus of elasticity $Ov$ overlap ratio, expressed as a percentage
$Ov, lim$	overlap limit for brace shear check
$N_0$	applied axial force in chord
$N_i$	applied axial force in brace $i$ ( $i = 1$ or $2$ )
$N_{pl,0}$	axial yield capacity of the chord
$N_u$	axial ultimate capacity
$M_0$	applied moment in chord
$M_{pl,0}$	plastic moment capacity of the chord
$M_u$	ultimate moment capacity
$M_{ip,1, Rd}$	design value of the in-plane moment in brace $i$ ( $i = 1$ or $2$ )
$V_{Ed}$	design value of the shear force in a chord member at the gap location
$V_{pl, Rd}$	design value of the shear force in a chord member
$V_s *$	design resistance of the joints, expressed in terms of the axial force in member $i$ ( $i = 1,2$ )
$L_0$	length of the chord in test set up
$L_1$	length of the brace in test set up

### Lowercases

$b_0$	external width of the chord
$b_i$	external width of brace $i$ ( $i = 1$ or $2$ )
$d_i$	external diameter of brace $i$ ( $i = 1$ or $2$ )
$h_0$	external height of the chord $h_i$ external depth of brace $i$ ( $i = 1$ or $2$ )



$hz$	distance between the centres of gravity of the effective parts of the RHS brace
$t_0$	flange thickness of an I or H section chord
$t_i$	wall thickness of CHS or RHS brace $i$ ( $i = 1$ ou $2$ )
$t_w$	web thickness of an I or H chord
$t_f$	flange thickness of an I or H chord
$\theta_i$	angle between the brace $i$ and the chord ( $i = 1$ or $2$ )
$g$	gap between the braces $r$ inside corner radius between the web and flanges of an I or H section
$\sigma_{yo}$	yield stress of the chord
$\sigma_{yi}$	yield stress of the brace $i$ ( $i = 1$ or $2$ )
$\sigma_u$	ultimate stress $\varepsilon$ strain $\nu$ Poisson's ratio
$b_{ei}$	effective width of an RHS brace
$d_{ei}$	effective width of a CHS brace $b_w$ effective width for the web of an I or H section
$d_w$	depth of the web of an I or H section
$\alpha$	factor used in the equation of $A_s$ $b_{e, ov}$ effective width of an overlapping RHS brace at the connection to the overlapped brace
$d_{e, ov}$	effective width of an overlapping CHS brace at the connection to the overlapped brace
$l_{b, eff}$	effective perimeter for local yielding of the overlapping brace
$\gamma_{M0}$	partial safety factor for the resistance of cross sections of any class
$\gamma_{M1}$	partial safety factor for the resistance of the elements to buckling
$\mu$	Rotation stiffness of a joint. $\mu = 1$ for initial rotation stiffness.
$A_s$	shear area of a chord member

## ABSTRACT

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Draglines, the giant single-bucket excavators in existence today, are used mostly for the removal of overburden in large scale surface mines. Normally the, draglines are more than 4000 tons in weight, with boom length ranging from 70 m to 110 m and bucket capacity ranging from 10 m<sup>3</sup> to 120 m<sup>3</sup>. The dragline boom is a key component in dragline operation. Different designs of booms are in practice to enhance the effective utilization of the dragline with minimum downtime due to major breakdown in boom. During dragline operational cycle, as bucket is directly attached to the boom point sheave, the boom is subjected to the loading and unloading loads cyclically. This cyclic loading and unloading produces high to very high stresses in the boom structure and may lead to boom failure.

The booms are constructed using hollow structural steel pipes. It consists of 3- 4 main chords and 5 -6 bracing members connected along the length of the boom. The bracing members form complex overlapping or non-overlapping joints known as boom clusters. These clusters are the complex joints, which may fail over a certain period of time.

Because the boom design incorporates number of joints, hence any design or analysis of boom structure remains incomplete without critical investigation of joints. Therefore, critical joints observed on the basis of field investigation have been chosen for the analysis of stresses within the joints. Four critical locations of the joints have been selected for in depth analysis of stresses around the interaction point of brace with the main chord. These joints have been chosen on the basis of field understanding that the repairing and maintenance activities are carried out frequently at these locations.

Two type of loading conditions, namely static and dynamic loading were observed. The static loading conditions occur during the operation, when the boom and bucket of the dragline become static for a very short period of time.

The dynamic loading conditions occur during the swing-to segment of dragline bucket cycle

with filled material for unloading the material from face to disposal site. It also occurs during the swing-back segment of dragline bucket cycle with empty bucket after dumping the material at the disposal site and returning back to the blasted face.

Accordingly, a solid 3D Model of the boom was constructed in SOLIDWORKS software as wireframe design. For performing simulations, the constructed model was imported to ANSYS software in compatible file format. Simulations were performed on ANSYS 18 using finite element analysis.

The overall structural behavior of the boom along with suspension rope, A-frame, and mast has been predicted by constructing and analyzing the structure as global beam model. The global beam model has provided the stresses in the form of direct or axial stress, bending stress and maximum combined stress on the boom structure. Deformation of the boom structure and tensile axial forces in the suspension wire ropes has also been predicted to ensure the safe functioning of the structure under the applied loading conditions. Further, the selected joints have been evaluated by analyzing the models as solid submodel. The solid sub-model has provided the Von-Mises stress; fatigue life and factor of safety in the boom cluster (joints). A separate parametric study has further been performed for the critical joint to predict stress level within this critical joint.

The design of dragline boom structure as a beam model and its analysis by using the state –of –art techniques has provided suitable insights in investigating the behavior of boom under static and dynamic loading conditions. The study has been able to propose a safe design of boom structure as the values obtained for the direct stresses, maximum bending stresses and maximum combined stresses are within the safe limits. The proposed design of the boom reveals that the main chord remains in axial compression or axial tension during the loading while bracing members are subjected to bending loads. The solid sub modeling of boom cluster (Joints) reveal that the stresses within the joints are concentrated at the fillet region. However,