Chapter 4

The Test Results and Discussions

4.1 Introduction

Different factors such as curing period, addition of super plasticizer, replacement of cement with Fly Ash, micro silica, GGBS, amongst others have been varied in test cube samples and the samples have been tested by the conventional phenolphthalein indicator method. Concrete samples with variable mix proportions as per experimental programme have been tested for carbonation depth by phenolphthalein indicator method at fixed parameters i.e. 3 days curing period, 4 % CO₂ concentration and 65 % relative humidity and retention period of 20 days in carbonation chamber at three different W/C ratios. Further, some samples have been tested by phenolphthalein indicator method upon variation of parameters like CO₂ concentration, curing period and RH percentage. However, some cases have also been tested by advanced methods for comparison of results.

Since the above variations also bear impact on the compressive strength of concrete which is the major characteristic of concrete in construction, compressive strength of specific samples under such variation have also been tested. An ideal set of parameters can be adjudged where the result of carbonation depth decreases without considerably affecting the compressive strength of concrete.

The following texts elaborate the experimental results obtained through various methods under various parameters.

4.2 Carbonation Depth from Phenolphthalein Indicator

4 % CO₂, 65 % RH and period of exposure 20 days

4.2.1 OPC Concrete (Without Super Plasticizer)

The results of the carbonation of the reference OPC concrete mixes, with W/C ratio varying over a range of 0.4 to 0.5 and cured for a period of 3, 7, 21, 28 and 56 days, before exposing them to a rapid carbonation environment in the laboratory, as stated above, are given in table 4.1 and shown plotted in Figure 4.1

Table 4.1 Carbonation Depth of OPC Concrete Mixes (W	Without Super Plasticizer)
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N	lix	W/C	Carbonation Depth (mm)					
			Period of Curing (Days)					
			3	7	21	28	56	
C) 1	0.4	4.4	4.1	4	3.9	3.7	
C) ₂	0.45	7.5	6.9	6.7	6.5	6.3	
C) ₃	0.5	10.1	9.1	8.8	8.5	8.3	



It is evident from test results that carbonation depth increases with increase in water cement ratio. This can be attributed to the increase in porosity of concrete with the increase of W/C, which subsequently facilitates the ingress of deleterious materials through the structure.

It may further be observed from the results that for OPC concrete, Carbonation depth decreases as the curing period increases. Longer curing period increases hydration of cement and decreases porosity, which in turn reduces the permeability of concrete and thus decreases penetration through carbonation.

4.2.2 OPC Concrete (With Super Plasticizer)

The results of the carbonation of the reference OPC concrete mixes, with W/C ratio varying over a range of 0.4 to 0.5 and cured for a period of 3, 7, 21, 28 and 56 days, before exposing them to a rapid carbonation environment in the laboratory, as stated above, are given in table 4.2 and shown plotted in Figure 4.2

Table 4.2 Carbonation Depth of OPC Concrete Mixes (With Super Plasticizer)

Mix	W/C		Carbonation Depth (mm)				
			P	eriod of Cur	ing (Days)		
		3	7	21	28	56	
OS ₁	0.4	3.8	3.6	3.5	3.4	3.2	
OS_2	0.45	6.3	6.1	5.7	5.6	5.5	
OS_3	0.5	8.7	8.2	7.7	7.4	7.2	



While it is established from the above results that carbonation depth increases with increase in water cement ratio and that carbonation depth decreases with increasing

curing period, it can also be seen from the above experimental data that use of super plasticizer reduces the effect of carbonation in concrete. Superplasticizers are applied to increase the workability of cement which thus increases its resistance to permeability and subsequently carbonation.

4.2.3 OPC Concrete with Fly Ash Replacement of Cement

The results of the carbonation of the reference OPC concrete mixes, with W/C ratio varying over a range of 0.4 to 0.5, with 10, 30, 50 and 70% replacement of cement with fly ash and cured for a period of 3 days, before exposing them to a rapid carbonation environment in the laboratory, as stated above, are given in table 4.3 and shown plotted in Figure 4.3

W/C	Percentage Replacement	Mix	Carbonation Depth (mm)
	No replacement of cement	O_1	4.4
	10 % replacement of cement with Fly Ash	OF ₃	3.9
0.4	30 % replacement of cement with Fly Ash	OF ₆	4.4
	50 % replacement of cement with Fly Ash	OF ₉	4.5
	70 % replacement of cement with Fly Ash	OF ₁₂	4.6
	No replacement of cement	O_2	7.5
	10 % replacement of cement with Fly Ash	OF ₂	7.1
0.45	30 % replacement of cement with Fly Ash	OF ₅	7.9
	50 % replacement of cement with Fly Ash	OF ₈	8.2
	70 % replacement of cement with Fly Ash	OF11	8.4
	No replacement of cement	O 3	10.1
	10 % replacement of cement with Fly Ash	OF ₁	9.7
0.5	30 % replacement of cement with Fly Ash	OF ₄	11.1
	50 % replacement of cement with Fly Ash	OF ₇	12
	70 % replacement of cement with Fly Ash	OF_{10}	12.5

Table 4.3 Carbonation Depth of OPC Concrete Mixes with Fly Ash Replacement of Cement



It can be observed from test results that for OPC concrete, depth of carbonation initially reduces up to 10 % replacement of cement with FA. However, increase in percentage replacement of cement with FA results in an increase of carbonation. This indicates that while lower percentages of replacement of cement with FA can considerably help filling the voids in concrete and reduce permeability thus reducing carbonation, higher replacement results in enhanced reduction of Ca(OH)₂ content of cement which reduces the basic environment of concrete mix and facilitates higher carbonation.

4.2.4 OPC Concrete with Micro Silica Replacement of Cement

The results of the carbonation of the reference OPC concrete mixes, with W/C ratio varying over a range of 0.4 to 0.5, with10, 30, 50 and 70% replacement of cement with Micro silica and cured for a period of 3 days, before exposing them to a rapid carbonation environment in the laboratory, as stated above, are given in table 4.4 and shown plotted in Figure 4.4

 Table 4.4 Carbonation Depth of OPC Concrete Mixes with Micro Silica Replacement of

 Cement

W/C	Percentage Replacement	Mix	Carbonation Depth (mm)
	No replacement of cement	O_1	4.4
	10 % replacement of cement with Micro silica	OM_3	3.9
0.4	30 % replacement of cement with Micro silica	OM_6	5.5
	50 % replacement of cement with Micro silica	OM ₉	5.8
	70 % replacement of cement with Micro silica	OM_{12}	5.9
	No replacement of cement	O_2	7.5
	10 % replacement of cement with Micro silica	OM_2	6.8
0.45	30 % replacement of cement with Micro silica	OM ₅	9.4
	50 % replacement of cement with Micro silica	OM ₈	10.1
	70 % replacement of cement with Micro silica	OM ₁₁	10.5
	No replacement of cement	O ₃	10.1
	10 % replacement of cement with Micro silica	OM_1	9.4
0.5	30 % replacement of cement with Micro silica	OM_4	12.6
	50 % replacement of cement with Micro silica	OM ₇	13.2
	70 % replacement of cement with Micro silica	OM_{10}	14.5



It can be observed from test results that for OPC concrete, depth of carbonation initially reduces up to 10 % replacement of cement with micro silica. However, increase in percentage replacement of cement with micro silica results in an increase of carbonation. This indicates that while lower percentages of replacement of cement with FA can considerably help filling the voids in concrete and reduce permeability thus reducing carbonation, higher replacement results in enhanced reduction of Ca(OH)₂ content of

cement which reduces the basic environment of concrete mix and facilitates higher carbonation.

4.2.5 OPC Concrete with Fly Ash Replacement of Fine Aggregate

The results of the carbonation of the reference OPC concrete mixes, with W/C ratio varying over a range of 0.4 to 0.5, with10, 30, 50 and 70% replacement of fine aggregate with fly ash and cured for a period of 3 days, before exposing them to a rapid carbonation environment in the laboratory, as stated above, are given in table 4.5 and shown plotted in Figure 4.5

Table 4.5 Carbonation depth of OPC Concrete Mixes with Fly Ash Replacement of Fine Aggregate

		C	Carbonation
W/C	Percentage Replacement	Mix	Depth
			(mm)
	No replacement of Fine aggregate	O_1	4.4
	10 % replacement of Fine aggregate with Fly Ash	OFF ₃	3.9
0.4	30 % replacement of Fine aggregate with Fly Ash	OFF ₆	3.4
	50 % replacement of Fine aggregate with Fly Ash	OFF ₉	3
	70 % replacement of Fine aggregate with Fly Ash	OFF ₁₂	2.8
	No replacement of Fine aggregate	O_2	7.5
	10 % replacement of Fine aggregate with Fly Ash	OFF ₂	6.6
0.45	30 % replacement of Fine aggregate with Fly Ash	OFF5	5.9
	50 % replacement of Fine aggregate with Fly Ash	OFF ₈	5.3
	70 % replacement of Fine aggregate with Fly Ash	OFF ₁₁	4.7
	No replacement of Fine aggregate	O ₃	10.1
	10 % replacement of Fine aggregate with Fly Ash	OFF_1	9.5
0.5	30 % replacement of Fine aggregate with Fly Ash	OFF ₄	8.5
	50 % replacement of Fine aggregate with Fly Ash	OFF ₇	8.2
	70 % replacement of Fine aggregate with Fly Ash	OFF ₁₀	8



It can be observed from the above test results that for OPC concrete, depths of carbonation decreases with increase in percentage replacement of fine aggregate with FA. It may be noted that results of cement replacement and fine aggregate replacement with FA do not follow the same pattern. Since fine aggregate does not contribute to cementitious properties and FA replacement of fine aggregate further reduces porosity, increased replacement consistently increases carbonation resistance in case of the latter.

4.2.6 OPC Concrete with GGBS Replacement of Cement

The results of the carbonation of the reference OPC concrete mixes, with W/C ratio varying over a range of 0.4 to 0.5, with10, 30, 50 and 70% replacement of cement with GGBS and cured for a period of 3 days, before exposing them to a rapid carbonation environment in the laboratory, as stated above, are given in table 4.6 and shown plotted in Figure 4.6

Table 4.6 Carbonation Depth of OPC Concrete Mixes with GGBS Replacement of Cement

W/C	Percentage Replacement	Mix	Carbonation Depth (mm)
		0	
	No replacement of cement	O_1	4.4
A (10 % replacement of cement with GGBS	OG₃	3.7
0.4	30 % replacement of cement with GGBS	OG_6	3.5
	50 % replacement of cement with GGBS	OG ₉	4.3
	70 % replacement of cement with GGBS	OG ₁₂	4.5
	No replacement of cement	O ₂	7.5
	10 % replacement of cement with GGBS	OG_2	6.9
0.45	30 % replacement of cement with GGBS	OG ₅	6.7
	50 % replacement of cement with GGBS	OG ₈	8
	70 % replacement of cement with GGBS	OG ₁₁	8.4
	No replacement of cement	O ₃	10.1
	10 % replacement of cement with GGBS	OG ₁	9.5
0.5	30 % replacement of cement with GGBS	OG_4	9.2
	50 % replacement of cement with GGBS	OG7	11.5
	70 % replacement of cement with GGBS	OG ₁₀	12



It can be observed from the above test results that for OPC concrete, carbonation depth initially decreases up to 30 % replacement of cement with GGBS and subsequently increases on increasing percentage replacement. This indicates that while lower percentages of replacement of cement with GGBS can considerably help filling the voids in concrete and reduce permeability thus reducing carbonation, higher replacement results

in enhanced reduction of $Ca(OH)_2$ content of cement which reduces the basic environment of concrete mix and facilitates higher carbonation.

It may further be noted that decrease in carbonation in case of cement replacement with GGBS is observed up to 30 % while that in case of FA replacement is up to 10 % only. This may be accounted for by the comparatively higher binding property of GGBS over FA on account of the Ca(OH)₂ content of the former.

4.2.7 OPC Concrete with GGBS Replacement of Fine Aggregate

The results of the carbonation of the reference OPC concrete mixes, with W/C ratio varying over a range of 0.4 to 0.5, with 10, 30, 50 and 70% replacement of fine aggregate with GGBS and cured for a period of 3 days, before exposing them to a rapid carbonation environment in the laboratory, as stated above, are given in table 4.7 and shown plotted in Figure 4.7

Table 4.7	Carbonation	Depth of	f OPC	Concrete	Mixes	with	GGBS	Replacement	of F	Fine
Aggregate	:									

W/C	Percentage Replacement	Mix	Carbonation Depth (mm)
	No replacement of Fine aggregate	O ₁	4.4
	10 % replacement of Fine aggregate with GGBS	OFG₃	3.8
0.4	30 % replacement of Fine aggregate with GGBS	OFG_6	3.2
	50 % replacement of Fine aggregate with GGBS	OFG ₉	2.9
	70 % replacement of Fine aggregate with GGBS	OFG_{12}	2.7
	No replacement of Fine aggregate	O ₂	7.5
	10 % replacement of Fine aggregate with GGBS	OFG_2	6.5
0.45	30 % replacement of Fine aggregate with GGBS	OFG₅	5.9
	50 % replacement of Fine aggregate with GGBS	OFG ₈	5.2
	70 % replacement of Fine aggregate with GGBS	OFG_{11}	4.8
		0	40.4
	No replacement of Fine aggregate	O_3	10.1
	10 % replacement of Fine aggregate with GGBS	OF_{G1}	9.4
0.5	30 % replacement of Fine aggregate with GGBS	OFG_4	8.5



It can be observed that for OPC concrete, carbonation reduces with increase in percentage replacement of fine aggregate with GGBS. It may be noted that results of cement replacement and fine aggregate replacement with GGBS do not follow the same pattern. Since fine aggregate does not contribute to cementitious properties and GGBS replacement of fine aggregate further reduces porosity, increased replacement consistently increases carbonation resistance in case of the latter.

4.2.8 Period of Exposure for OPC Concrete

The results of the carbonation of the reference OPC concrete mixes, with W/C ratio varying over a range of 0.4 to 0.5 with 20, 60, 90, 120, 150 days period of exposure and cured for a period of 3 days, before exposing them to a rapid carbonation environment in the laboratory, as stated above, are given in table 4.8 and shown plotted in Figure 4.8

Table 4.8 Carbonation Depths of OPC Concrete Mixes for Various Periods of Exposure

Mix	W/C	Carbonation Depth (mm)				
			Period o	f Exposure (Days)	
		20	60	90	120	150
O ₁	0.4	4.4	8.6	10.1	10.8	12.2
O ₂	0.45	7.5	13.2	14.2	17.3	18.2
O ₃	0.5	10.1	15.5	23.8	26.8	30.1



It can be observed from the above experimental results that for OPC concrete, the carbonation depth increases with the period of exposure. This indicates that the impact of carbonation progressively increases with time.

4.2.9 External Cover of OPC Concrete

The results of the carbonation of the reference OPC concrete mixes, with W/C ratio varying over a range of 0.4 to 0.5 for concrete and concrete with 12 mm plaster/acrylic emulsion paint and cured for a period of 3 days, before exposing them to a rapid carbonation environment in the laboratory, as stated above, are given in table 4.9 and shown plotted in Figure 4.9

Table 4.9 Carbonation Depth vs. OPC Concrete with/without Plaster and Paint

Mix	W/C	Carbonation				
		Depth				
		(mm)				
		Concrete	Concrete with	Concrete with 12 mm		
			12 mm plaster	plaster and acrylic		
				emulsion paint		

		Depth (mm)						
		Concrete	Concrete with 12 mm plaster	Concrete with 12 mm plaster and acrylic emulsion paint				
O 1	0.4	4.4	3.2	2.5				
O ₂	0.45	7.5	5.5	4.8				
O 3	0.5	10.1	7.2	5.2				



It can be seen from the above experimental results that for OPC concrete, carbonation depth reduces with increase in external protective cover. External covers like paint and plaster offer resistance to entry of deleterious materials and thus reduces the vulnerability of concrete to deterioration from carbonation.

4.2.10 PPC Concrete (Without Super Plasticizer)

The results of the carbonation of the reference PPC concrete mixes, with W/C ratio varying over a range of 0.4 to 0.5 and cured for a period of 3, 7, 21, 28 and 56 days, before exposing them to a rapid carbonation environment in the laboratory, as stated above, are given in table 4.10 and shown plotted in Figure 4.10

Mix	W/C	Carbonation Depth (mm)					
		Period of Curing (Days)					
	_	3	7	21	28	56	
P ₁	0.4	8.2	7.4	7.3	7.1	6.8	
P ₂	0.45	14.1	12.7	12.1	11.8	11.6	
P ₃	0.5	18.8	16.8	16.2	15.8	15.2	

Table 4.10 Carbonation Depth of PPC Concrete Mixes (Without Super Plasticizer)



It is evident from test results that carbonation depth in PPC concrete increases with increase in water cement ratio and that carbonation depth decreases as the curing period increases, as already seen in case of OPC concrete also.

It is also observed that the depths of carbonation in PPC concrete are higher than that of OPC concrete. The addition of SCM to clinker in case of PPC reduces the Clinker content in PPC thus lowering the available content of Ca(OH)₂. As a result, less CH content is available for hydration to CaCO₃. CaCO₃ being voluminous compound that contributes to reduction in pore space plays a major role in porosity of concrete. Thus formation of

comparatively less volume of CaCO₃ in PPC results in more porous concrete facilitating higher ingress of CO₂ and thus increased carbonation.

4.2.11 PPC Concrete (With Super Plasticizer)

The results of the carbonation of the reference PPC concrete mixes, with W/C ratio varying over a range of 0.4 to 0.5 and cured for a period of 3, 7, 21, 28 and 56 days, before exposing them to a rapid carbonation environment in the laboratory, as stated above, are given in table 4.11 and shown plotted in Figure 4.11

Table 4.11 Carbonation D	pth of PPC Concrete Mixes	(With Super Plasticizer)
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Mix	w/C	Carbonation Depth (mm)				
		Period of Curing (Days)				
		3	7	21	28	56
PS₁	0.4	6.8	6.7	6.5	6.3	6.1
PS_2	0.45	11.5	11.1	10.6	10.3	10.1
PS₃	0.5	16	15.2	14.1	13.4	13.2



It can be established from the above results that carbonation depth increases with increase

in water cement ratio and that carbonation depth decreases with increasing curing period as well as the fact that carbonation in PPC concrete is higher than that in OPC concrete. It can also be seen from the above experimental data that use of super plasticizer reduces the effect of carbonation in concrete just as that in OPC as observed above.

4.2.12 PPC Concrete with Fly Ash Replacement of Cement

The results of the carbonation of the reference PPC concrete mixes, with W/C ratio varying over a range of 0.4 to 0.5, with10, 30, 50 and 70% replacement of cement with fly ash and cured for a period of 3 days, before exposing them to a rapid carbonation environment in the laboratory, as stated above, are given in table 4.12 and shown plotted in Figure 4.12

W/C	Percentage Replacement	Mix	Carbonation Depth (mm)
	No replacement of compart	Р	0.0
	10 replacement of cement		0.2
0.4	10 % replacement of cement with Fly Ash		1.2
0.4	30 % replacement of cement with Fly Ash	PF_6	8.4
	50 % replacement of cement with Fly Ash	PF ₉	8.5
	70 % replacement of cement with Fly Ash	PF_{12}	8.6
	No replacement of cement	P	1/1
	10 % replacement of compart with Ely Ach		19.1
0.45	10 % replacement of cement with Fix Ash		12.1
0.45	30 % replacement of cement with Fly Ash		14.8
	50 % replacement of cement with Fly Ash	PF ₈	15.2
	70 % replacement of cement with Fly Ash	PF ₁₁	15.3
	No replacement of cement	P.	18.8
	10.0% replacement of compart with Fly Ach		17.0
0.5	10 % replacement of cement with Fly Ash		17.4
0.5	30 % replacement of cement with Fly Ash	PF_4	22.1
	50 % replacement of cement with Fly Ash	PF_7	23
	70 % replacement of cement with Fly Ash	PF ₁₀	23.2

Table 4.12 Carbonation Depth of PPC Concrete Mixes with Fly Ash Replacement of Cement



It can be observed from test results that for PPC concrete also, depth of carbonation initially reduces up to 10 % replacement of cement with FA. However, increase in percentage replacement of cement with FA results in an increase of carbonation for similar reasons as observed in case of OPC concrete above.

4.2.13 PPC Concrete with Micro Silica Replacement of Cement

The results of the carbonation of the reference PPC concrete mixes, with W/C ratio varying over a range of 0.4 to 0.5, with 10, 30, 50 and 70% replacement of cement with micro silica and cured for a period of 3 days, before exposing them to a rapid carbonation environment in the laboratory, as stated above, are given in table 4.13 and shown plotted in Figure 4.13

Carbonatic	on
Cement	
Table 4.13 Carbonation Depth of PPC Concrete Mixes with Micro Silica Replacement of	2

W/C	Percentage Replacement	Mix	Carbonation Depth (mm)
	No replacement of cement	P ₁	8.2
0.4	10 % replacement of cement with Micro silica	PM ₃	5.8
	30 % replacement of cement with Micro silica	PM_6	10.1
	50 % replacement of cement with Micro silica	PM ₉	11.2
	70 % replacement of cement with Micro silica	PM ₁₂	11.4

	No replacement of cement	P_2	14.1
	10 % replacement of cement with Micro silica	PM ₂	10.1
0.45	30 % replacement of cement with Micro silica	PM ₅	17.4
	50 % replacement of cement with Micro silica	PM ₈	18.7
	70 % replacement of cement with Micro silica	PM ₁₁	19.5
	No replacement of cement	P ₃	18.8
	10 % replacement of cement with Micro silica	PM ₁	15.1
0.5	30 % replacement of cement with Micro silica	PM ₄	23.5
	50 % replacement of cement with Micro silica	PM ₇	24.5
	70 % replacement of cement with Micro silica	PM ₁₀	25.9



It can be observed from test results that for PPC concrete also, depth of carbonation initially reduces up to 10 % replacement of cement with micro silica. However, increase in percentage replacement of cement with micro silica results in an increase of carbonation for reasons as observed in case of OPC concrete above.

4.2.14 PPC Concrete with Fly Ash Replacement of Fine Aggregate

The results of the carbonation of the reference PPC concrete mixes, with W/C ratio varying over a range of 0.4 to 0.5, with 10, 30, 50 and 70% replacement of fine aggregate with fly ash and cured for a period of 3 days, before exposing them to a rapid carbonation environment in the laboratory, as stated above, are given in table 4.14 and shown plotted in Figure 4.14

Table 4.14 Carbonation Depth of PPC Concrete Mixes with Fly Ash Replacement of Fine Aggregate

		C	arbonation
W/C	Percentage Replacement	Mix	Depth (mm)
	No replacement of Fine aggregate	P ₁	8.2
	10 % replacement of Fine aggregate with Fly Ash	PFF_3	6.2
0.4	30 % replacement of Fine aggregate with Fly Ash	PFF_6	5.8
	50 % replacement of Fine aggregate with Fly Ash	PFF ₉	5.4
	70 % replacement of Fine aggregate with Fly Ash	PFF_{12}	5.3
	No replacement of Fine aggregate	P ₂	14.1
	10 % replacement of Fine aggregate with Fly Ash	PFF_2	11.2
0.45	30 % replacement of Fine aggregate with Fly Ash	PFF₅	10.8
	50 % replacement of Fine aggregate with Fly Ash	PFF ₈	10.1
	70 % replacement of Fine aggregate with Fly Ash	PFF ₁₁	9.7
	No replacement of Fine aggregate	P ₃	18.8
	10 % replacement of Fine aggregate with Fly Ash	PFF ₁	14.2
0.5	30 % replacement of Fine aggregate with Fly Ash	PFF_4	13.1
	50 % replacement of Fine aggregate with Fly Ash	PFF ₇	12.2
	70 % replacement of Fine aggregate with Fly Ash	PFF_{10}	11.9



It can be observed from the above test results that for PPC concrete, depths of carbonation decreases with increase in percentage replacement of fine aggregate with FA. It may be

noted that results of cement replacement and fine aggregate replacement with FA do not follow the same pattern.

4.2.15 PPC Concrete with GGBS Replacement of Cement

The results of the carbonation of the reference PPC concrete mixes, with W/C ratio varying over a range of 0.4 to 0.5, with10, 30, 50 and 70% replacement of cement with GGBS and cured for a period of 3 days, before exposing them to a rapid carbonation environment in the laboratory, as stated above, are given in table 4.15 and shown plotted in Figure 4.15

Table 4.15 Carbonation Depth of PPC Concrete Mixes with GGBS Replacement of Cement

W/C	Percentage Replacement	Mix	Carbonation Depth (mm)
	No replacement of cement	P1	8.2
	10 % replacement of cement with GGBS	PG3	7
0.4	30 % replacement of cement with GGBS	PG6	6.5
	50 % replacement of cement with GGBS	PG9	8.4
	70 % replacement of cement with GGBS	PG12	8.6
	No replacement of cement	P2	14.1
	10 % replacement of cement with GGBS	PG2	12
0.45	30 % replacement of cement with GGBS	PG5	11.7
	50 % replacement of cement with GGBS	PG8	15
	70 % replacement of cement with GGBS	PG11	15.5
	No replacement of cement	P3	18.8
	10 % replacement of cement with GGBS	PG1	17.3
0.5	30 % replacement of cement with GGBS	PG4	16.9
	50 % replacement of cement with GGBS	PG7	22.5
	70 % replacement of cement with GGBS	PG10	23



It can be observed from the above test results that for PPC concrete, carbonation depth initially decrease up to 30 % replacement of cement with GGBS and subsequently increases on increasing percentage replacement due to similar reasons as observed in case of OPC concrete above.

4.2.16 PPC Concrete with GGBS Replacement of Fine Aggregate

The results of the carbonation of the reference PPC concrete mixes, with W/C ratio varying over a range of 0.4 to 0.5, with10, 30, 50 and 70% replacement of fine aggregate with GGBS and cured for a period of 3 days, before exposing them to a rapid carbonation environment in the laboratory, as stated above, are given in table 4.16 and shown plotted in Figure 4.16

Table 4.16	Carbonation	Depth	of PPC	Concrete	Mixes	with	GGBS	replacement	of l	Fine
Aggregate										

W/C	Percentage Replacement	Mix	Carbonation Depth (mm)
	No replacement of Fine aggregate	P1	8.2
	10 % replacement of Fine aggregate with GGBS	PFG3	6.2
0.4	30 % replacement of Fine aggregate with GGBS	PFG6	5.9
	50 % replacement of Fine aggregate with GGBS	PFG9	5.5
	70 % replacement of Fine aggregate with GGBS	PFG12	5.2

0.45	No replacement of Fine aggregate	P2	14.1
	10 % replacement of Fine aggregate with GGBS	PFG2	11
	30 % replacement of Fine aggregate with GGBS	PFG5	10.6
	50 % replacement of Fine aggregate with GGBS	PFG8	10
	70 % replacement of Fine aggregate with GGBS	PFG11	9.8
0.5	No replacement of Fine aggregate	P3	18.8
	10 % replacement of Fine aggregate with GGBS	PFG1	14
	30 % replacement of Fine aggregate with GGBS	PFG4	12.8
	50 % replacement of Fine aggregate with GGBS	PFG7	12
	70 % replacement of Fine aggregate with GGBS	PFG10	11.7



It can be observed that for PPC concrete also, carbonation reduces with increase in percentage replacement of fine aggregate with GGBS for similar reasons explained in case of OPC concrete above.

4.2.17 Period of Exposure of PPC Concrete

The results of the carbonation of the reference OPC concrete mixes, with W/C ratio varying over a range of 0.4 to 0.5 with 20, 60, 90, 120 and 150 days period of exposure and cured for a period of 3 days, before exposing them to a rapid carbonation environment in the laboratory, as stated above, are given in table 4.17 and shown plotted in Figure 4.17

Mix	W/C	Carbonation Depth (mm) Period of Exposure (Days)				
		10	20	30	40	50
P ₁	0.4	5.4	8.2	10.4	11.2	12.9
P ₂	0.45	9.1	14.1	16.9	18.8	20.3
P ₃	0.5	12.9	18.8	25.9	27.8	32.6

Table 4.17 Carbonation Depth of PPC Concrete Mixes for Various Periods of Exposure



It can be seen from the above experimental results that for PPC concrete also, carbonation depth increases with increase in period of exposure.

4.2.18 External Cover of PPC Concrete

The results of the carbonation of the reference PPC concrete mixes, with W/C ratio varying over a range of 0.4 to 0.5 for 12mm plaster and acrylic emulsion paint and cured for a period of 3 days, before exposing them to a rapid carbonation environment in the laboratory, as stated above, are given in table 4.18 and shown plotted in Figure 4.1

Mix	W/C	Concrete	Concrete with 12 mm Plaster	Concrete with 12 mm Plaster and Acrylic Emulsion Paint
O ₁	0.4	8.2	6.8	5.9
O ₂	0.45	14.1	11.1	9.5
O ₃	0.5	18.8	14.8	11.8

Table 4.18 Carbonation Depth vs. PPC Concrete with/without Plaster and Paint



It can be seen from the above experimental results that for PPC concrete also, carbonation depth reduces with increase in external protective covers like paint and plaster which offer resistance to entry of deleterious materials and thus reduces the vulnerability of concrete to deterioration from carbonation.

4.3 Carbonation Depth from Phenolphthalein Indicator at Variable Parameters

4.3.1 CO₂ Concentration on OPC Concrete

The results of the carbonation of the reference OPC concrete mixes, with W/C ratio varying over a range of 0.4 to 0.5 with 4,10,20,30 and 50 % of CO₂ and cured for a period of 3 days, before exposing them to a rapid carbonation environment in the laboratory, as stated above, are given in table 4.19 and shown plotted in Figure 4.19

Mix	W/C	Carbonation Depth (mm)				
		Percentage of CO ₂ (%)				
		4	10	20	30	50
O ₁	0.4	4.4	7.5	9.9	12.1	17.8
O ₂	0.45	7.5	11.8	16.3	20.6	26.7
O ₃	0.5	10.1	16.1	21.6	27.5	34.1

Table 4.19 Carbonation Depth of OPC Concrete Mixes for various percentages of CO2



It can be seen from the experimental results above that for OPC concrete, carbonation depth increases with increase in percentage of CO_2 concentration. This indicates that rate of carbonation depends on the rate of diffusion of CO_2 into the concrete sample.

4.3.2 CO₂ Concentration on PPC Concrete

The results of the carbonation of the reference PPC concrete mixes, with W/C ratio varying over a range of 0.4 to 0.5 with 4, 10, 20, 30 and 50 % of CO_2 and cured for a period of 3 days, before exposing them to a rapid carbonation environment in the laboratory, as stated above, are given in table 4.20 and shown plotted in Figure 4.20

Mix	W/C		Carbonation Depth (mm)			
		Percentage of CO ₂ (%)				
		4	10	20	30	50
P ₁	0.4	8.2	11	19.4	20.5	31.1
P ₂	0.45	14.1	20.2	35.5	42.1	55.6
P ₃	0.5	18.8	35.2	45.1	50.6	70.1



It can be seen that for PPC concrete also, carbonation depth increases with increase in percentage of CO_2 concentration for reasons cited above.

4.3.3 Relative Humidity on OPC Concrete

The results of the carbonation of the reference OPC concrete mixes, with W/C ratio varying over a range of 0.4 to 0.5 with 40, 50, 65, 85 and 100 % of Relative Humidity and cured for a period of 3 days, before exposing them to a rapid carbonation environment in the laboratory, as stated above, are given in table 4.21 and shown plotted in Figure 4.21

Mix	W/C	Carbonation Depth (mm)				
		Percentage of Relative Humidity (%)			6)	
		40	50	65	85	100
O ₁	0.4	3.9	4.1	4.4	4.4	4.2
O ₂	0.45	6.5	6.9	7.5	7.5	6.5
O ₃	0.5	9.2	9.5	10.1	10.2	9.1

 Table 4.21 Carbonation Depth of OPC Concrete Mixes for Various Percentages of

 Relative Humidity



It can be seen that for OPC concrete, carbonation initially increases with an increase in RH up to 85 % and then subsequently decreases with increase of RH. The increase in RH increases the moisture content and fluidity of particles as well as transportation of CO_2 within the pores thus facilitating progressive carbonation. However, after 85 % RH, the pores are fully saturated with moisture and thus resist the transportation of CO_2 through the pores.

4.2.4 Relative Humidity on PPC Concrete

The results of the carbonation of the reference PPC concrete mixes, with W/C ratio varying over a range of 0.4 to 0.5 with 40, 50, 65, 85 and 100 % of Relative Humidity and cured for a period of 3 days, before exposing them to a rapid carbonation environment in the laboratory, as stated above, are given in table 4.22 and shown plotted in Figure 4.22

 Table 4.22 Carbonation Depth of PPC Concrete Mixes for Various Percentages of

 Relative Humidity

Mix	W/C	Carbonation Depth (mm)				
		Perc	entage o	f Relative	e Humidit	t y (%)
		40	50	65	85	100
P ₁	0.4	7.1	7.9	8.2	8.21	7.6
P ₂	0.45	10	12.7	14.1	14.5	12.2
P ₃	0.5	14.2	17.6	18.8	19.1	15.2



It can be seen that for PPC concrete also, carbonation initially increases with an increase in RH up to 85 % and then subsequently decreases with increase of RH for reasons cited above.

4.4 Carbonation Depth from XRD Analysis

The CaCO₃ peak lies in the 2 Θ value of (29.37). The same also confirms to JCPDS pdf no. 72-1937.

Carbonation depth is obtained by X- Ray diffraction pattern at different depths of concrete cubes. A significant reduction in peak height of $CaCO_3$ can be observed with increasing the depth of sample.

Fig. 4.23 to 4.28 shows XRD Graph between intensity and diffraction angle (2 Θ) of different types of concrete at 3 days curing period, 4 % CO₂ concentration and 65 % relative humidity and retention period of 20 days in carbonation chamber for both OPC and PPC concrete. Q represents peak value of intensity for quartz (SiO₂) and C represents peak value of intensity for CaCO₃. The presence of peak value for CaCO₃ in the graphs indicate carbonation at given depths while the absence of peak indicates no carbonation at the given depth since CaCO₃ which is a resultant of carbonation is not present at that depth.





Fig 4.23 XRD RESULTS FOR OPC CONCRETE AT W/C 0.4

A study of XRD analysis of sliced concrete samples randomly at depths 4.4 mm, 8.8 mm and 9.1 mm for 2 Θ values corresponding to CaCO₃ (C) and SiO₂ (Q) represented in Figure 4.23 above reveals that the peak value of intensity for 2 Θ values corresponding to CaCO₃ is observed in graphs at 4.4. mm sample and 8.8 mm sample but not in 9.1 mm sample. The presence of peak values of intensity at 2 Θ values corresponding to CaCO₃ indicates the presence of the compound formed as a result of carbonation at that depth. The absence of peak value at 9.1 mm indicates absence of CaCO₃ at that depth which indicates that carbonation has not taken place at that particular depth.

Thus depth of carbonation can be ascertained as 8.8 mm.





Fig 4.24 XRD RESULTS FOR OPC CONCRETE AT W/C 0.45

A study of XRD analysis of sliced concrete samples randomly at depths 7.5 mm, 15.0 mm and 15.5 mm for 2 Θ values corresponding to CaCO₃ (C) and SiO₂ (Q) represented in Figure 4.24 above reveals that the peak value of intensity for 2 Θ values corresponding to CaCO₃ is observed in graphs at 7.5 mm sample and 15.0 mm sample while extremely small peak is observed at 15.5 mm. Thus it can be inferred that depth of carbonation lies between 15.0 mm and 15.5 mm i.e. approximately 15.2 mm.

4.4.3 OPC Concrete with W/C 0.5



Fig 4.25 XRD RESULTS FOR OPC CONCRETE AT W/C 0.5

A study of XRD analysis of sliced concrete samples randomly at depths 10.1 mm, 19.1 mm and 20.2 mm for 2 Θ values corresponding to CaCO₃ (C) and SiO₂ (Q) represented in

Figure 4.25 above reveals that the peak value of intensity for 2 Θ values corresponding to CaCO3 is observed in graphs at 10.1 mm sample and 19.1 mm sample while extremely small peak is observed at 20.2 mm. Thus it can be inferred that depth of carbonation is approximately 19.4 mm.

4.4.4 PPC Concrete with W/C 0.4



Fig 4.26 XRD RESULTS FOR PPC CONCRETE AT W/C 0.4

A study of XRD analysis of sliced concrete samples randomly at depths 8.2 mm, 14.0 mm and 16.5 mm for 2 Θ values corresponding to CaCO3 (C) and SiO2 (Q) represented in Figure 4.26 above reveals that the peak value of intensity for 2 Θ values corresponding to CaCO3 is observed in graphs at 8.2 mm sample and 14.0 mm sample but not in 16.5 mm sample. Thus depth of carbonation can be ascertained as 14.0 mm.

4.4.5 PPC Concrete with W/C 0.45



Fig 4.27 XRD RESULTS FOR PPC CONCRETE AT W/C 0.45

A study of XRD analysis of sliced concrete samples randomly at depths 14.1 mm, 28.2 mm and 30.0 mm for 2 Θ values corresponding to CaCO3 (C) and SiO2 (Q) represented in Figure 4.27 above reveals that the peak value of intensity for 2 Θ values corresponding to CaCO₃ is observed in graphs at 14.1 mm sample and 28.2 mm sample but not in 30.0 mm sample. Thus depth of carbonation can be ascertained as 28.2 mm.

4.4.6 PPC Concrete with W/C 0.5



Fig 4.28 XRD RESULTS FOR PPC CONCRETE AT W/C 0.5

A study of XRD analysis of sliced concrete samples randomly at depths 18.8 mm, 37.7 mm and 40.0 mm for 2 Θ values corresponding to CaCO3 (C) and SiO2 (Q) represented in Figure 4.25 above reveals that the peak value of intensity for 2 Θ values corresponding to CaCO₃ is observed in graphs at 18.8 mm sample and 37.7 mm sample while extremely small peak is observed at 40.0 mm. Thus depth of carbonation lies between 37.7 mm and 40.0 mm. It can be inferred that depth of carbonation is 38.8 mm.

4.5 Results Obtained From FTIR Method

In FTIR method, Carbonation depth can be derived from a study of the characteristic peak of the C-O functional group in the wavenumber range of 1410-1510 cm⁻¹. Carbonation depth is determined from the position of the C-O characteristic peaks relative to the baseline. Graphs between absorbance and wavenumber for different type of concrete with different W/C at 3 days curing period, 4 % CO₂ concentration and 65 % relative humidity and retention period of 20 days in carbonation chamber for both OPC and PPC concrete are shown in Fig. 4.29 to Fig. 4.34 and the depth of carbonation is derived with reference to a baseline at 37 % transmittance as per Chang et al (2006). Peaks lying below the baseline indicate depth of carbonation.

4.5.1 OPC Concrete with W/C 0.4



Fig 4.29 FTIR RESULTS FOR OPC CONCRETE AT W/C 0.4

From Figure 4.29 it can be inferred that depth of carbonation in concrete sample of OPC cement with 0.4 W/C is 9.4 mm, since the trough values lying in the range of Wave number 1410-1510 cm⁻¹ falls below the base line for each of the graphs pertaining to depths of 4.4 mm, 8.8 mm and 9.4 mm. Since 9.4 mm just lies at the baseline, depth of carbonation is concluded at 9.4 mm.

4.5.2 OPC Concrete with W/C 0.45



Fig 4.30 FTIR RESULTS FOR OPC CONCRETE AT W/C 0.45

From Figure 4.30 it can be inferred that depth of carbonation in concrete sample of OPC cement with 0.45 W/C is 15.3 mm, since the trough values lying in the range of Wave

number 1410-1510 cm⁻¹falls below the base line for each of the graphs pertaining to depths of 7.5 mm and 15.0 mm and lies just above the baseline for the graph pertaining to 15.5 mm. Hence, it is inferred that the depth of carbonation is 15.3 mm.

4.5.3 OPC Concrete with W/C 0.5



Fig 4.31 FTIR RESULTS FOR OPC CONCRETE AT W/C 0.5

From Figure 4.31 it can be inferred that depth of carbonation in concrete sample of OPC cement with 0.5 W/C is 19.7 mm, since the trough values lying in the range of Wave number 1410-1510 cm⁻¹falls below the base line for each of the graphs pertaining to depths of 10.1 mm and 19.1 mm and lies above the baseline for 20.2 mm. Since the

baseline is in between the graphs for 19.1 mm and 20.2 mm, it can be inferred that depth of carbonation is 19.7 mm.

4.5.4 PPC Concrete with W/C 0.4



Fig 4.32 FTIR RESULTS FOR PPC CONCRETE AT W/C 0.4

From Figure 4.32 it can be inferred that depth of carbonation in concrete sample of PPC cement with 0.4 W/C is 16.3 mm, since the trough values lying in the range of Wave number 1410-1510 cm⁻¹falls below the base line for each of the graphs pertaining to depths of 8.25 mm and 14.0 mm and lies just above the baseline for the graph pertaining to 16.5 mm. Hence, it can be inferred that the depth of carbonation is 16.3 mm.

4.5.5 PPC Concrete with W/C 0.45



Fig 4.33 FTIR RESULTS FOR PPC CONCRETE AT W/C 0.45

From Figure 4.33 it can be inferred that depth of carbonation in concrete sample of PPC cement with 0.45 W/C is 29.4 mm, since the trough values lying in the range of Wave number 1410-1510 cm⁻¹falls below the base line for each of the graphs pertaining to depths of 14.1 mm and 28.2 mm and lies above the baseline for 30.0 mm. Since the baseline is in between the graphs for 28.2 mm and 30.0 mm, it can be inferred that depth of carbonation is 29.4 mm.





Fig 4.34 FTIR RESULTS FOR PPC CONCRETE AT W/C 0.5

From Figure 4.34 it can be inferred that depth of carbonation in concrete sample of PPC cement with 0.5 W/C is 34.0 mm, since the trough values lying in the range of Wave number 1410-1510 cm⁻¹falls below the base line for the graph pertaining to depths of 18.85 mm and lies above the baseline for each of the graphs pertaining to 36.0 mm and 37.75 mm. Since the graph corresponding to 36.0 mm depth is near the baseline, it can be inferred that depth of carbonation is 34.0 mm.

4.6 Results Obtained From SEM-EDX Analysis

SEM-EDX Analysis were done on concrete samples with different W/C at 3 days curing period, 4 % CO₂ concentration, 65 % relative humidity and retention period of 20 days in carbonation chamber for both OPC and PPC concrete. The SEM images and corresponding EDX analysis reflects the presence of products like calcium, oxygen and carbon, owing to the presence of calcite. Since calcite results from carbonation, SEM EDX analysis at different depths of the concrete test sample indicates the depth of carbonation.

Results obtained are as follows:

4.6.1 OPC concrete at W/C 0.4

4.6.1(a) Tests results at 8.8 mm



Element	Weight%	Atomic%
ск	23.02	34.12
ок	42.41	47.19
Mg K	8.58	6.28
Si K	4.49	2.85
Ca K	21.51	9.55
Totals	100.00	

4.6.1(b) Tests results at 9.2 mm



C K 24.26 35.81 O K 41.00 45.43 Si K 17.96 11.33 Ca K 16.79 7.42 Totals 100.00

Weight%

Atomic%

Element

4.6.1(c) Tests results at 11.0 mm



Inference : Carbon content as per above images and data is found at 8.8 mm and 9.2 mm depths but not at 11.0 mm and beyond. From the above it can be concluded that carbonation depth for OPC concrete at W/C 0.4 is 9.2 mm since calcium carbonate is found at depth 8.8 mm and 9.2 mm but not at 11.0 mm and beyond.

4.6.2 OPC concrete at W/C 0.45

4.6.2 (a) Tests results at 17.0 mm



Element	Weight%	Atomic%
ок	50.47	66.78
Al K	10.35	8.12
Si K	19.57	14.75
Ca K	19.61	10.35
Totals	100.00	

4.6.2(b) Tests results at 16.0 mm



Element	Weight%	Atomic%
ок	53.43	71.15
Si K	18.04	13.69
Ca K	28.53	15.16
Totals	100.00	

4.6.2(c) Test results at 14.1 mm



Inference : Carbon content as per above images and data is found at 14.1 mm depths but not at 16.0 mm and 17.0 mm. From the above it can be concluded that carbonation depth for OPC concrete at W/C 0.45 is 14.1 mm since calcium carbonate is found at depths 14.1 mm but not at 16.0 mm and 17.0 mm.

4.6.3 OPC concrete at W/C 0.5

4.6.3(a) Test results at 23.0 mm



Element	Weight%	Atomic%
ок	65.25	79.05
Mg K	3.58	2.85
Si K	14.67	10.12
Ca K	16.50	7.98
Totals	100.00	

4.5.3(b) Test results at 21.0 mm



Element	Weight%	Atomic%
ок	55.22	71.49
ΑΙ Κ	8.02	6.16
Si K	15.24	11.24
Ca K	21.52	11.12
Totals	100.00	

4.5.3(c) Test results at 18.7 mm



Inference : Carbon content as per above images and data is found at 18.7 mm depths but not at 21.0 mm and 23.0 mm. From the above it can be concluded that carbonation depth for OPC concrete at W/C 0.5 is 18.7 mm since calcium carbonate is found at depths 18.7 mm but not at 21.0 mm and 23.0 mm.

4.6.4 PPC concrete at W/C 0.4

4.6.4(a) Test results at 16.0 mm



Element	Weight%	Atomic%	
ок	57.24	74.78	
ΑΙ Κ	1.96	1.52	
Si K	10.86	8.08	
Ca K	29.93	15.61	
Totals	100.00		

4.6.4(b) Test results at 15.0 mm



7µm Electron Image 1

Element	Weight%	Atomic%	
ок	48.94	66.39	
AI K	4.04	3.25	
Si K	20.83	16.10	
КК	6.54	3.63	
Ca K	19.64	10.64	
Totals	100.00		

4.6.4(c) Test results at 13.1 mm



Element	Weight%	Atomic%	
СК	21.35	32.62	
ок	43.12	49.44	
AI K	1.13	0.77	
Si K	8.11	5.29	
Ca K	25.11	11.49	
Fe K	1.19	0.39	
Totals	100.00		

Inference : Carbon content as per above images and data is found at 13.1 mm depths but not at 15.0 mm and 16.0 mm. From the above it can be concluded that carbonation depth for PPC concrete at W/C 0.4 is 13.1 mm since calcium carbonate is found at depths 13.1 mm but not at 15.0 mm and 16.0 mm.

4.6.5 PPC concrete at W/C 0.45

4.6.5 (a) Test results at 30.0 mm



Element	Weight%	Atomic%	
ок	44.09	63.74	
ΑΙ Κ	5.98	5.13	
Si K	9.37	7.71	
Ca K	40.56	23.41	
Totals	100.00		

4.6.5(b) Test results at 28.0 mm



Element	Weight%	Atomic%	
ок	16.36	28.72	
Si K	4.21	4.21	
SK	69.59	60.96	
Ca K	5.81	4.07	
Fe K	4.03	2.03	
Totals	100.00		

4.6.5(c) Test results at 26.9 mm



Element	Weight%	Atomic%	
СК	24.85	35.92	
ОК	44.83	48.64	
Mg K	1.40	1.00	
Al K	1.81	1.16	
Si K	9.61	5.94	
Ca K	15.55	6.73	
Fe K	1.96	0.61	
Totals	100.00		

Inference : Carbon content as per above images and data is found at 26.9 mm depths but not at 28.0 mm and 30.0 mm. From the above it can be concluded that carbonation depth for PPC concrete at W/C 0.45 is 26.9 mm since calcium carbonate is found at depths 26.9 mm but not at 28.0 mm and 30.0 mm.

4.6.6 PPC concrete atW/C 0.5

4.6.6(a) Test results at 40.0 mm



Element	Weight%	Atomic%	
ок	42.54	62.70	
Mg K	1.85	1.79	
Si K	19.14	16.07	
кк	1.98	1.19	
Ca K	27.49	16.17	
Br L	7.01	2.07	
Totals	100.00		

4.6.6(b) Test results at 38.0 mm



Weight% Atomic% Element о к 48.41 66.08 Mg K 2.43 2.18 AI K 4.82 3.91 Si K 15.76 12.25 Ca K 28.59 15.58 100.00 Totals

4.6.6(c) Test results at 36.7 mm



Element	Weight%	Atomic%	
СК	40.23	64.31	
ОК	0.23	0.28	
Si K	0.58	0.40	
SK	57.05	34.16	
Ca K	1.41	0.68	
Fe K	0.49	0.17	
Totals	100.00		

Inference : Carbon content as per above images and data is found at 36.7 mm depths but not at 38.0 mm and 40.0 mm. From the above it can be concluded that carbonation depth for PPC concrete at W/C 0.5 is 36.7 mm since calcium carbonate is found at depths 36.7 mm but not at 38.0 mm and 40.0 mm.

4.7 Comparison of Carbonation Depth Obtained From Various Methods

4.7.1Comprasion of carbonation depth of OPC Concrete obtained from Phenolphthalein, XRD,

SEM EDX and FTIR nethod are, given in table 4.23 and shown plotted in Figure 4.35

 Table 4.23 - Comparison of Carbonation of OPC Concrete Obtained using various

 methods

Mix	W/C	Carbonation Depth (mm)			
		Phenolphthalein	XRD	SEM EDX	FTIR
O ₁	0.4	4.4	8.8	9.2	9.4
O ₂	0.45	7.5	15.2	14.1	15.3
O ₃	0.5	10.1	19.4	18.7	19.7



4.7.2 Comprasion of Carbonation Depth of PPC Concrete obtained from Phenolphthalein, XRD,

SEM EDX and FTIR Method are, given in table 4.24 and plotted in Figure 4.36.

Mix	W/C	Carbonation Depth (mm)				
		Phenolphthalein	XRD	SEM EDX	FTIR	
P ₁	0.4	8.2	14	13.1	16.3	
P_2	0.45	14.1	28.2	26.9	29.4	
P ₃	0.5	18.85	38.9	36.7	34	



From the above figures it can be inferred that the carbonation depth obtained from advance methods are much higher than those obtained from conventional phenolphthalein indicator method, while the results obtained from different advanced methods i.e. XRD, FTIR and SEM are nearly comparable.

4.8 Comparison of Carbonation Depth of OPC Conrete and PPC Concrete

4 % CO₂ and 65 % Relative Humidity

Carbonation Depths of OPC Conrete and PPC Concrete at 4 % CO₂ and 65 % Relative

Humidity for variable periods of exposure are given in table 4.25

	Mix	W/C	Carbonation Depth (mm)			
		-		Period of Exposure (days)		
			3	7	21	28
	O ₁	0.4	4.4	4.1	4	3.9
OPC	O ₂	0.45	7.5	6.9	6.7	6.5
	O ₃	0.5	10.1	9.1	8.8	8.5
	P ₁	0.4	8.25	7.45	7.35	7.15
PPC	P_2	0.45	14.1	12.77	12.15	11.88

Table 4.25 Carbonation Depth of OPC Concrete vs. PPC Concrete

P ₃	0.5	18.85	16.84	16.25	15.85

From the above table, it can be clearly seen that carbonation depth for PPC concrete are much higher than those in OPC concrete.

4.9 Conclusions

The following can be concluded from the above results

- a. Use of OPC is recommended against PPC to reduce the risk of carbonation.
- b. The use of superplasticizers reduces the effect of carbonation in both OPC and PPC concrete and also reduces cement consumption.
- c. In areas of high CO₂ concentration, protective covers such as acrylic emulsion paints over plaster must be ensured.
- d. Replacement of fine aggregate with fly ash/GGBS is recommended for both OPC and PPC concrete to reduce the effect of carbonation Replacement of cement with fly ash or micro silica up to 10% is recommended for reduction in carbonation only i.e. Replacement of OPC cement with fly ash or micro silica is recommended for reduction in carbonation
- e. Replacement of OPC cement with up to 30% of GGBS is recommended for reduction in carbonation.
- f. Longer curing period is recommended for both OPC and PPC concrete to reduce the risk of carbonation.
- g. Carbonation depth obtained from advance methods are higher than those obtained from conventional phenolphthalein indicator method, while the results obtained from different advanced methods i.e. XRD, FTIR and SEM are nearly comparable.

- h. It can be seen that for both OPC and PPC concrete also, carbonation initially increases with an increase in RH up to 85 % and then subsequently decreases with increase of RH
- i. Carbonation depth increases with increase in percentage of CO_2 concentration for both OPC and PPC concrete. This indicates that rate of carbonation depends on the rate of diffusion of CO_2 into the concrete sample.